

Project: Plan 2800

Location: Garage Roof Girder Truss Footing
Footing

[2015 International Building Code(2012 NDS)]

Footing Size: 3.0 FT x 3.0 FT x 10.00 IN

Reinforcement: #4 Bars @ 9.00 IN. O.C. E/W / (4) min.

Section Footing Design Adequate

BASIC PERMIT PACKAGE
REVIEWED FOR CODE COMPLIANCE
WITH IRC 2015
KITSAP COUNTY BUILDING DEPARTMENT

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#[18-02763]



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StruCalc Version 9.0.2.5

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FOOTING PROPERTIES

Allowable Soil Bearing Pressure: $Q_s = 1500$ psf
Concrete Compressive Strength: $F'_c = 2500$ psi
Reinforcing Steel Yield Strength: $F_y = 60000$ psi
Concrete Reinforcement Cover: $c = 3$ in

FOOTING SIZE

Width: $W = 3$ ft
Length: $L = 3$ ft
Depth: $\text{Depth} = 10$ in
Effective Depth to Top Layer of Steel: $d = 6.25$ in

COLUMN AND BASEPLATE SIZE

Column Type: Wood
Column Width: $m = 6$ in
Column Depth: $n = 6$ in

FOOTING CALCULATIONS

Bearing Calculations:

Ultimate Bearing Pressure: $Q_u = 1111$ psf
Effective Allowable Soil Bearing Pressure: $Q_e = 1375$ psf
Required Footing Area: $A_{req} = 7.27$ sf
Area Provided: $A = 9.00$ sf

Baseplate Bearing:

Bearing Required: $Bear = 14800$ lb
Allowable Bearing: $Bear-A = 99450$ lb

Beam Shear Calculations (One Way Shear):

Beam Shear: $V_{u1} = 4831$ lb
Allowable Beam Shear: $V_{c1} = 16875$ lb

Punching Shear Calculations (Two Way Shear):

Critical Perimeter: $B_o = 49$ in
Punching Shear: $V_{u2} = 13086$ lb
Allowable Punching Shear (ACI 11-35): $vc2-a = 68906$ lb
Allowable Punching Shear (ACI 11-36): $vc2-b = 81563$ lb
Allowable Punching Shear (ACI 11-37): $vc2-c = 45938$ lb
Controlling Allowable Punching Shear: $vc2 = 45938$ lb

Bending Calculations:

Factored Moment: $M_u = 66600$ in-lb
Nominal Moment Strength: $M_n = 251888$ in-lb

Reinforcement Calculations:

Concrete Compressive Block Depth: $a = 0.62$ in
Steel Required Based on Moment: $A_s(1) = 0.20$ in²
Min. Code Req'd Reinf. Shrink./Temp. (ACI-10.5.4): $A_s(2) = 0.65$ in²
Controlling Reinforcing Steel: $A_{s-reqd} = 0.65$ in²
Selected Reinforcement: #4's @ 9.0 in. o.c. e/w (4) Min.
Reinforcement Area Provided: $A_s = 0.79$ in²

Development Length Calculations:

Development Length Required: $L_d = 15$ in
Development Length Supplied: $L_{d-sup} = 15$ in

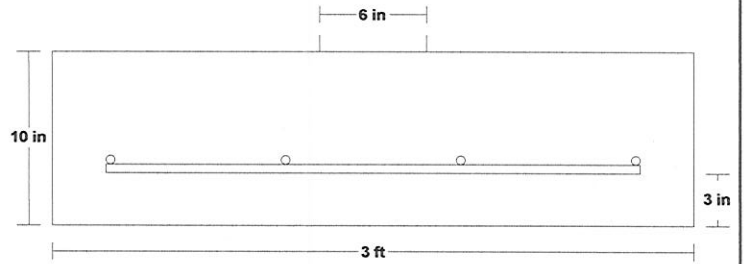
NOTES

LOADING DIAGRAM

Subject To Field Inspection

Reviewed for code compliance
with IRC 2015
Kitsap County Building Department
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07/21/2020

CHANGES
MUST Be Approved Prior
To Performing Work



FOOTING LOADING

Live Load: $PL = 7000$ lb
Dead Load: $PD = 3000$ lb
Total Load: $PT = 10000$ lb
Ultimate Factored Load: $P_u = 14800$ lb
Weight to resist uplift w/ 1.5 F.S.: $U.R. = 725$ lb



Project: Plan 2800

Location: Garage Floor Support om Stem Wall

Footing

[2015 International Building Code(2012 NDS)]

Footing Size: 16.0 IN Wide x 6.0 IN Deep Continuous Footing With 6.0 IN Thick x 24.0 IN Tall Stemwall

LongitudinalReinforcement: (2) Continuous #4 Bars

TransverseReinforcement: #4 Bars @ 18.00 IN. O.C. (unnecessary)

Section Footing Design Adequate



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FOOTING PROPERTIES

Allowable Soil Bearing Pressure: $Q_s = 1500$ psf
Concrete Compressive Strength: $F'_c = 2500$ psi
Reinforcing Steel Yield Strength: $F_y = 60000$ psi
Concrete Reinforcement Cover: $c = 3$ in

FOOTING SIZE

Width: $W = 16$ in
Depth: $\text{Depth} = 6$ in
Effective Depth to Top Layer of Steel: $d = 2.25$ in

STEMWALL SIZE

Stemwall Width: 6 in
Stemwall Height: 24 in
Stemwall Weight: 150 pcf

FOOTING CALCULATIONS

Bearing Calculations:

Ultimate Bearing Pressure: $Q_u = 563$ psf
Effective Allowable Soil Bearing Pressure: $Q_e = 1425$ psf
Width Required: $W_{req} = 0.53$ in²

Beam Shear Calculations (One Way Shear):

Beam Shear: $V_{u1} = 182$ lb
Allowable Beam Shear: $V_{c1} = 2025$ lb

Transverse Direction:

Bending Calculations:

Factored Moment: $M_u = 828$ in-lb
Nominal Moment Strength: $M_n = 0$ in-lb

Reinforcement Calculations:

Concrete Compressive Block Depth: $a = 0.30$ in
Steel Required Based on Moment: $A_s(1) = 0.01$ in²
Min. Code Req'd Reinf. Shrink./Temp. (ACI-10.5.4): $A_s(2) = 0.13$ in²
Controlling Reinforcing Steel: $A_{s-reqd} = 0.13$ in²
Selected Reinforcement: Trans: #4's @ 18.0 in. o.c.
Reinforcement Area Provided: $A_s = 0.13$ in²

Development Length Calculations:

Development Length Required: $L_d = 15$ in
Development Length Supplied: $L_{d-sup} = 2$ in

Note: Plain concrete adequate for bending, therefore adequate development length not required.

Longitudinal Direction:

Reinforcement Calculations:

Min. Code Req'd Reinf. Shrink./Temp. (ACI-10.5.4): $A_s(2) = 0.17$ in²
Controlling Reinforcing Steel: $A_{s-reqd} = 0.17$ in²
Selected Reinforcement: Longitudinal: (2) Cont. #4 Bars
Reinforcement Area Provided: $A_s = 0.39$ in²

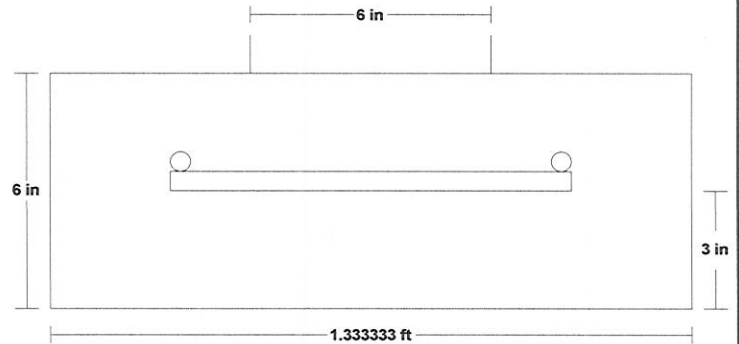
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FOOTING LOADING

Live Load: $PL = 400$ lb
Dead Load: $PD = 200$ lb
Total Load: $PT = 750$ lb
Ultimate Factored Load: $P_u = 1060$ lb

